

## PRELIMINARY FOUNDATION ENGINEERING REPORT

# **LEICESTER MIDDLE SCHOOL**

# LEICESTER, MASSACHUSETTS

## **APRIL 1, 2019**

Prepared For:

Finegold Alexander Architects 77 North Washington Street Boston, MA 02114

**PROJECT NO. 6743.2.00** 

2269 Massachusetts Avenue Cambridge, MA 02140 www.mcphailgeo.com (617) 868-1420



April 1, 2019

Finegold Alexander Architects 77 North Washington Street Boston, MA 02114

Attention: Ms. Regan Shields Ives

Reference: Leicester Middle School; Leicester, Massachusetts Preliminary Foundation Engineering Report

Ladies and Gentlemen:

This report documents the results of our subsurface exploration program and preliminary foundation design study to be included as part of a feasibility study of the planned construction of a new middle school building at the site of the existing Leicester Middle and High Schools located in Leicester, Massachusetts. Refer to the Project Location Plan (**Figure 1**) for the general site location.

This report was prepared in accordance with our proposal dated January 30, 2019, and the subsequent authorization of Finegold Alexander Architects (FAA). These services are subject to the limitations contained in **Appendix A**.

### Purpose and Scope

The purpose of our preliminary design study was to obtain initial subsurface information across the proposed building site and to identify preliminary foundation design considerations associated with the feasibility study assessing options for the proposed project.

#### **Available Information**

Information available to McPhail Associates, LLC (McPhail) for use in the preparation of this report included the following:

- An undated compilation of existing site survey information transmitted to McPhail electronically on February 27, 2019 from FAA; and
- A draft Existing Conditions Plan provided by Nitsch Engineering on April 1, 2019.

Elevations referenced herein are in feet and are referenced to the North American Vertical Datum of 1988 (NAVD 88).



#### **Existing and Proposed Conditions**

It is understood that a feasibility study is being conducted to assess several alternative locations for construction of a new middle school in the area north of the existing Leicester Middle School, which fronts onto Winslow Avenue to the south, and south of the existing Leicester High School, which fronts onto Paxton Street to the west. The existing school buildings are generally surrounded by bituminous concrete paved parking lots and roadways with landscaped margins. Grassed athletic fields occupy the majority of the remaining site area and a pond is located at the southeast corner of the site. The remainder of the site is generally bordered by wooded areas and/or residential properties. The Leicester Senior Center borders the site to the southeast.

It is understood that the proposed construction will likely include a new middle school which would be located in the general vicinity of the existing athletic fields. The size and layout of the proposed building is presently unknown, as well as if it would contain occupied below-grade space.

The athletic fields are generally separated into four relatively level areas which are considered as feasible options for the location of the proposed new school building. East of the existing high school building, the football field is generally level at about Elevation +1000. Located to the north of the existing middle school building, the softball field and general use field to the northwest of the softball field backstop is relatively level at approximately Elevation +995. The general use field consists of a slight slope from north to south down to approximately Elevation +991, where a more pronounced slope leads down to the lacrosse/soccer field at approximately Elevation +987. To the northeast of the existing middle school building, downhill of the surrounding asphalt parking lot, the baseball field is located at approximately Elevation +967.

#### Subsurface Exploration Program

A subsurface exploration program consisting of eight (8) borings was conducted at the site on March 5 and 6, 2019 by Technical Drilling Services (TDS) of Sterling, Massachusetts under contract to McPhail. Boring logs prepared by McPhail are contained in **Appendix B** and approximate plan locations of the borings are as indicated on the enclosed Subsurface Exploration Plan, **Figure 2**.

Borings were performed utilizing track-mounted drilling equipment. Each boring was advanced using 2.25-inch inner diameter hollow stem augers. Standard 2-inch O.D. split-spoon samples and standard penetration tests (SPT) were generally obtained at 5-foot intervals of depth in accordance with the standard procedures in ASTM D1586. The borings were terminated at depths ranging from 11.5 to 27 feet below the existing ground surface.

The explorations were observed by a representative of McPhail who performed field layout, prepared field logs, obtained and visually classified soil samples, monitored groundwater



conditions in the borings, and made minor adjustments to the exploration locations and determined the required exploration depths based upon the actual subsurface conditions encountered.

Field locations of the borings and the ground surface elevation at each boring location were determined by survey by Nitsch Engineering.

#### Laboratory Testing

At the completion of the subsurface exploration program, soil samples were returned to our laboratory for more detailed classification, analysis, and testing. The laboratory testing consisted of sieve analyses to determine the grain size distribution and confirm the visual classifications of the fill and glacial till deposits. Laboratory test procedures were in general accordance with applicable ASTM Standards. Results of the gradation testing appear on **Figure 3** and **Figure 4** following the text of this report.

#### **Subsurface Conditions**

A detailed description of the subsurface conditions encountered within the borings is documented on the boring logs contained in **Appendix B**. Based on these explorations, the following is a description of the generalized subsurface conditions encountered across the site from ground surface downward.

Underlying a thin surficial layer of topsoil, the borings encountered fill soil which extends to depths of about 2 to 7 feet below ground surface. The fill generally consists of a loose to dense, dark brown to orange-brown silt and sand with trace to some gravel varying to a gravelly sand with trace to some silt. The fill also contains trace amounts of root matter and clay and likely contains cobbles as well. Furthermore, the fill within boring MA-6 was also observed to contain a trace of brick and ash. Grain size distributions of samples of the fill are shown on **Figure 3**. Based on a comparison of the grain size distributions of the fill and underlying glacial till deposit, the fill observed in the borings appears to primarily consist of reworked natural glacial till.

A historic topsoil/subsoil layer was encountered underlying the fill within boring MA-1 at an approximate depth of 4 feet below the existing ground surface, extending to the glacial till deposit at a depth of approximately 6 feet. The historic topsoil/subsoil layer was generally observed to consist of a loose, black-brown sandy silt with trace gravel and some root matter.

Underlying the fill and/or historic topsoil/subsoil, a natural glacial till deposit was encountered within each boring at depths of 2 to 7 feet below grade, specifically ranging from about Elevation +998.6 at boring MA-7 to about Elevation +961.5 at boring MA-1. The glacial till deposit was observed to generally consist of a compact to very dense, brown



to orange-brown to gray silt and sand with trace to some gravel and trace clay varying to a silty sand with some to trace gravel and trace clay. Grain size distributions of samples of the glacial till deposit are shown on **Figure 4**.

Borings MA-1, MA-2, and MA-6 through MA-8 were terminated in the glacial till deposit at depths of 12 to 27 feet below ground surface. Borings MA-3 through MA-5 were terminated upon auger or split spoon refusal, which is generally assumed to be indicative of cobbles or boulders within the glacial till deposit or potentially the underlying bedrock surface, at approximate depths varying from 11.5 to 14.7 feet below ground surface.

Groundwater was observed in borings MA-1 and MA-3 through MA-6 upon completion of drilling at approximate depths ranging from about 6 to 8 feet below ground surface, corresponding to levels ranging from about Elevation +961.5 to about Elevation +992.6. It is anticipated that water levels could be indicative of groundwater that is perched on top of the relatively impervious glacial till deposit or the surface of the bedrock. Groundwater was not encountered in boring MA-2 upon completion of drilling. Additionally, it is anticipated that future groundwater levels across the site may vary from those reported herein due to factors such as normal seasonal changes, periods of heavy precipitation and alterations of existing drainage patterns.

#### **Preliminary Foundation Design Recommendations**

Based on the scope of the proposed development and the subsurface conditions encountered at the site, for preliminary design purposes it is recommended that foundation support for the proposed structure consist of conventional spread footing foundations in conjunction with slab-on-grade construction. Additional foundation design recommendations are contained below.

#### Footing Recommendations

Footings are recommended to bear on the natural, undisturbed glacial till deposit, or on imported gravel borrow that is placed and compacted over the natural glacial till deposit. For preliminary design purposes, the footings should be proportioned utilizing a maximum design bearing pressure of two (2) tons per square-foot (tsf). All foundations should be designed in accordance with the Code. Recommended minimum footing widths for continuous and isolated spread footings are 24 and 36 inches, respectively.

All footings in unheated areas should be provided with a minimum 4-foot thickness of soil cover as frost protection. Interior foundations should be located such that the top of the foundation concrete is a minimum of 6 inches below the underside of the lowest level slab. All foundations should be located such that they bear below a theoretical line drawn upward and outward at 2 to 1 (horizontal to vertical) from the bottom exterior edge of all adjacent footings, structures and utilities.



Fill material should be removed at footing locations and within the lateral limits defined herein for the placement of gravel borrow. Where proposed footings are to be supported on gravel borrow, the lateral limits of the excavation should extend beyond the outside edge of the footings for a horizontal distance equal to the depth from the bottom of the proposed footing to the surface of the natural, undisturbed glacial till deposit, plus two (2) feet in all plan directions.

Gravel borrow should consist of an off-site well-graded natural sand and gravel containing less than eight (8) percent passing the no. 200 sieve. Reuse of the on-site soil as ordinary fill outside the building footprint is discussed in more detail in the "Preliminary Geotechnical Construction Considerations" section of this report.

All gravel borrow placed within the footprint of the proposed building for support of the footings and slab-on-grade should be placed in lifts having a compacted thickness of 6 inches and be compacted to a minimum of 95 percent of its maximum modified Proctor dry density. The placement and compaction of gravel borrow should be monitored by a Registered Professional Engineer or his designated representative in accordance with the provisions of the Code.

#### Slab Recommendations

The proposed lowest level slabs should be designed as conventional soil-supported slabs-on-grade bearing on proof-compacted existing fill material or on imported gravel borrow that is placed and compacted over the proof-compacted existing fill material. Preparation of the building pad for support of the spread footings and slabs should include the removal of all topsoil from the entire proposed building footprint.

The existing fill, where encountered, may remain in place below the lowest level slab provided it is proof-compacted with a minimum of six (6) passes of a large walk-behind double drum vibratory roller. All soft, spongy or "weaving" areas observed during the proof-compaction should be removed and replaced with compacted gravel borrow.

The lowest level slabs should be underlain by a polyethylene vapor barrier spread across the surface of a 9-inch thickness of compacted ¾-inch crushed stone, which is underlain by filter fabric, such as Mirafi 140N or equivalent, spread across the proof-compacted fill or glacial till subgrade.

As indicated above, groundwater was encountered in several borings at depths of 6 to 8 feet below ground surface upon the completion of drilling. If the proposed lowest level slabs will be located below-grade, groundwater and/or surface water runoff that infiltrates into the ground could become periodically or seasonally perched on the surface of the fill or glacial till and infiltrate into the occupied below-grade space. Therefore, to protect the lowest level slabs from groundwater intrusion, underslab and perimeter foundation drains may be required. The proposed grading plan should be provided to McPhail for review to determine



if foundation drainage is required. Recommendations for foundation drainage, if required, would be contained in the Final Foundation Engineering Report (FFER).

All localized depressions in the lowest level slab (such as elevator pits, etc.) should be provided with properly tied continuous waterstops in all construction joints and cementitious waterproofing to protect against groundwater intrusion. Furthermore, the perimeter below-grade foundation walls should receive a trowelled-on bitumastic damproofing.

#### General Foundation Recommendations

Below-grade foundation walls receiving lateral support at the top and bottom (i.e. restrained walls) should be designed for a lateral earth pressure corresponding to an equivalent fluid density of 60 pounds per cubic-foot. Similarly, drained cantilevered retaining walls, (i.e. receiving no lateral support at the top) should be designed for a lateral earth pressure corresponding to an equivalent fluid density of 40 pounds per cubic-foot for a level backfill condition. To these values must be added the pressures attributable to earthquake forces per Section 1610.2 of the Code.

Cantilevered site retaining walls should be backfilled with free-draining material and provided with weep holes spaced at maximum 10-foot centers. Crushed stone surrounded by filter fabric should be provided at each weep hole.

Lateral forces can be considered to be transmitted from the structure to the soil by passive pressure against the foundation walls utilizing an equivalent fluid density of 120 pounds per cubic-foot providing that the walls are designed to resist these pressures. Lateral force can also be considered to be transmitted from the structures to the soil by friction on the base of footings using a coefficient of 0.35, to which a safety factor of 1.5 should be applied.

#### Seismic Design Considerations

For the purposes of determining parameters for structural seismic design, for preliminary design purposes this site is considered to be a Site Class D as defined in Chapter 20 of American Society of Civil Engineers (ASCE) Standard 7-10 "Minimum Design Loads for Buildings and Other Structures". The bearing strata on the proposed site are not considered to be subject to liquefaction during an earthquake based on the criterion of Section 1806.4 of the Code.

#### **Preliminary Geotechnical Construction Considerations**

The primary geotechnical construction considerations that are anticipated to have an impact on the design of the structure include the elevation of the proposed lowest level floor slab(s) in relation to the elevation of the surface of the natural glacial till deposit, and on-site reuse of excavated soils. Additional geotechnical construction considerations, such as preparation



of foundation and slab bearing surfaces, construction dewatering, and off-site removal of excess excavated material, should be discussed in the FFER.

As indicated above, the proposed footings are recommended to bear on the natural glacial till deposit or on compacted gravel borrow placed over the natural glacial till deposit. The existing fill located below the footings, and within the zone of influence of the footings, will need to be excavated and imported gravel borrow placed and compacted for support of the footings.

Depending on the location of the proposed building and the elevation of the lowest level slab(s), cuts and/or fills may be required to facilitate the building construction. To minimize the amount of imported gravel borrow that is required, it is recommended that the proposed finished slab elevations be located close to the existing site grades. If the site grades will be raised by more than a couple feet, consideration could be given to reusing the on-site fill soil as ordinary fill within the building footprint to raise the proposed grades and employing a ground improvement method such as aggregate piers (APs) to improve the characteristics of the fill in lieu of excavating the fill below footings and importing gravel borrow. As a ground improvement technique, APs are considered to be a technically suitable alternate to the placement of gravel borrow for foundation support. Furthermore, the structural design of the footings and slabs-on-grade supported on soil improved by APs would be the same as if gravel borrow were used.

As described above, grain size distributions of representative samples of the fill material indicate that the fines content (i.e. silt and clay) ranges from about 35 to 46 percent. In addition, grain size distributions of representative samples of the glacial till deposit indicate that the fines content ranges from about 41 to 51 percent. Due to the fines content of the on-site soils, excavated material may become unsuitable for re-use if it is not covered and becomes too wet to be properly compacted. Furthermore, when the on-site material is wet it is susceptible to freezing which would also prevent it from being acceptable for on-site reuse for support of the building foundations. If the earthwork operations are performed during a wet and/or cold period, it is anticipated that significant portions of the on-site soil may become unsuitable for re-use and slabs.

As such, at the present time the on-site fill and glacial till are not recommended to be reused on-site for support of the proposed footings or slabs (unless ground improvement methods are employed) due to the high fines content. It is anticipated that portions of the excavated soils may be re-used on-site as ordinary fill, provided they are maintained in a dry condition and can be properly compacted.

It is emphasized that excavated material will become unsuitable for re-use if it becomes too wet. Therefore, it is recommended that stockpiles of excavated material intended for reuse be protected against increases in moisture content by securely covering the stockpiles at all times with 6-mil polyethylene for protection from precipitation and also as a dust mitigation measure. The placement and compaction of on-site material should be completed during



relatively dry and non-freezing conditions. If, due to any of the above conditions, the excavated material is unsuitable for reuse, an off-site gravel borrow should be used.

### **Final Comments**

It is recommended that McPhail be retained to prepare a Final Foundation Engineering Report once the details of the proposed building project are finalized. The final report would provide final foundation recommendations based on the specific project design requirements. Additional subsurface explorations will be necessary to further delineate the subsurface conditions across the final building site.

We trust that the above preliminary information is sufficient for your present requirements. Should you have any questions concerning the recommendations presented herein, please do not hesitate to call us.

Very truly yours,

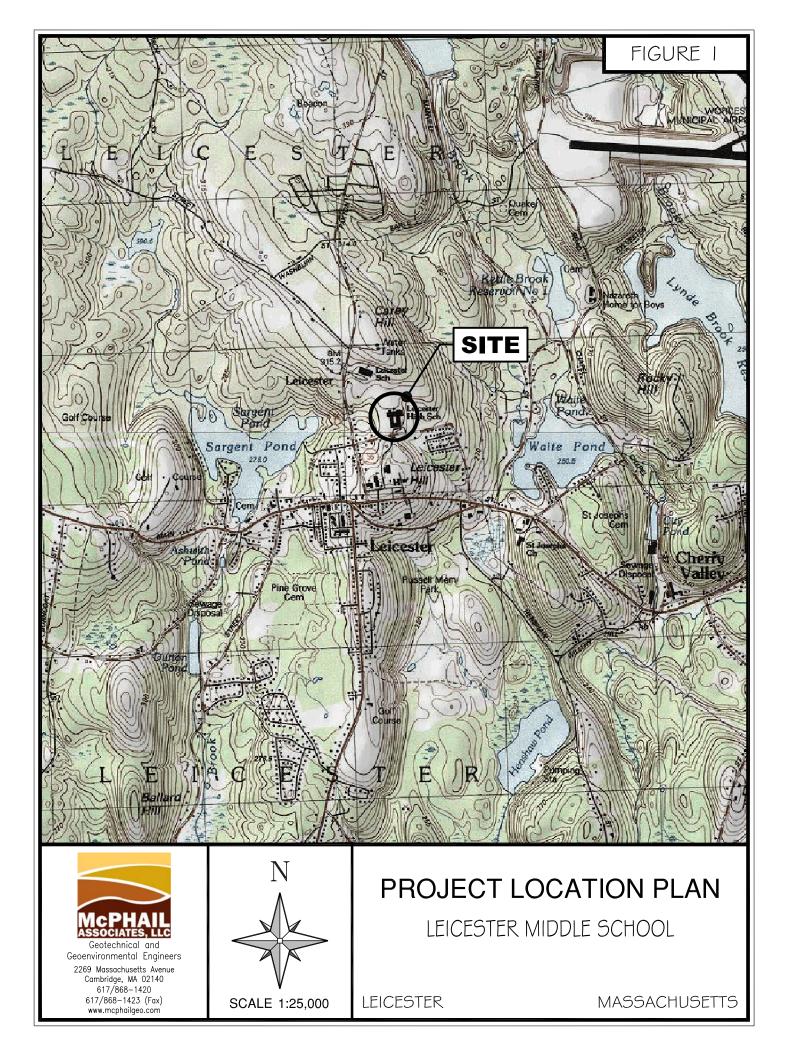
McPHAIL ASSOCIATES, LLC

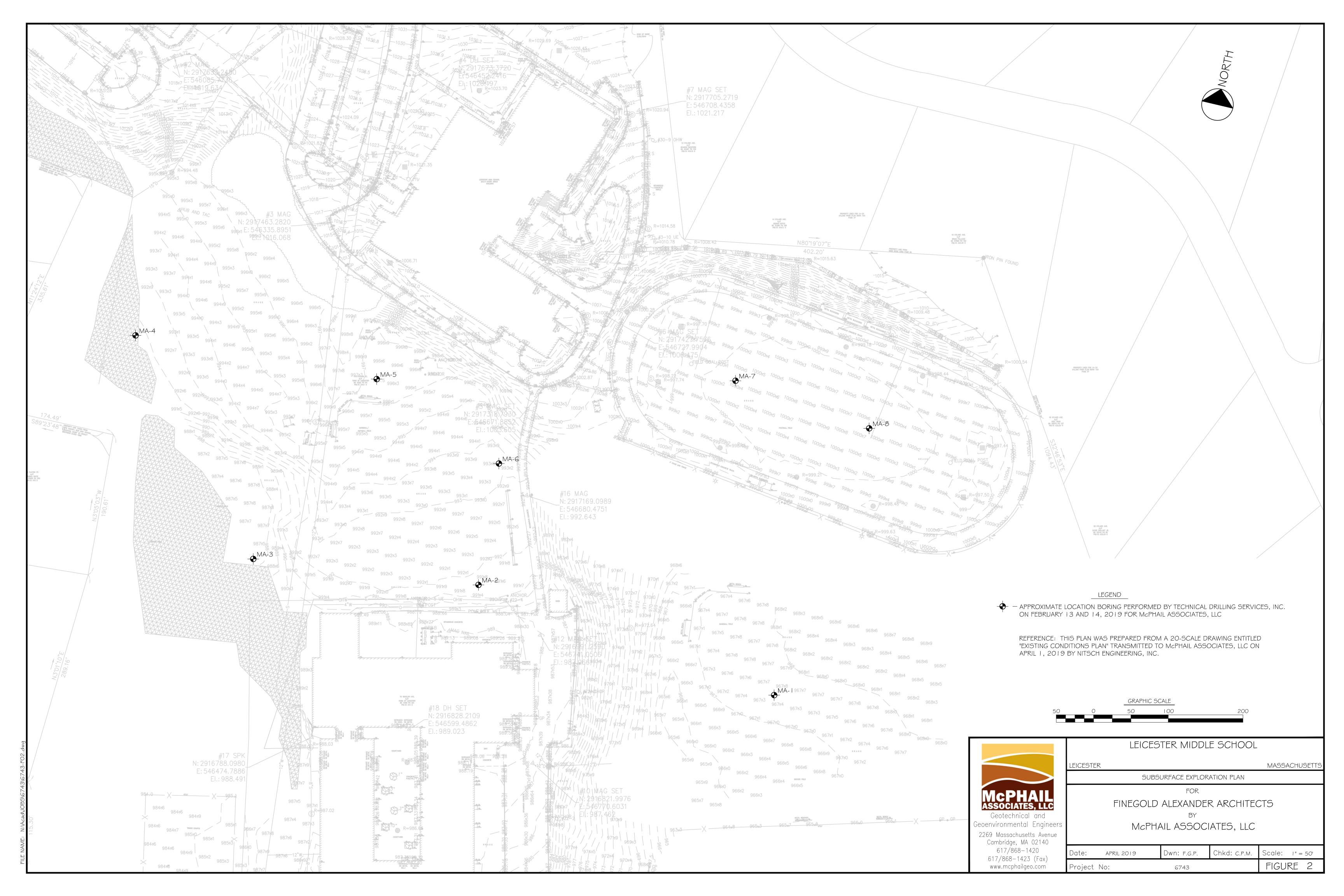
Christopher P. Miller

Jonathan W. Patch, P.E.

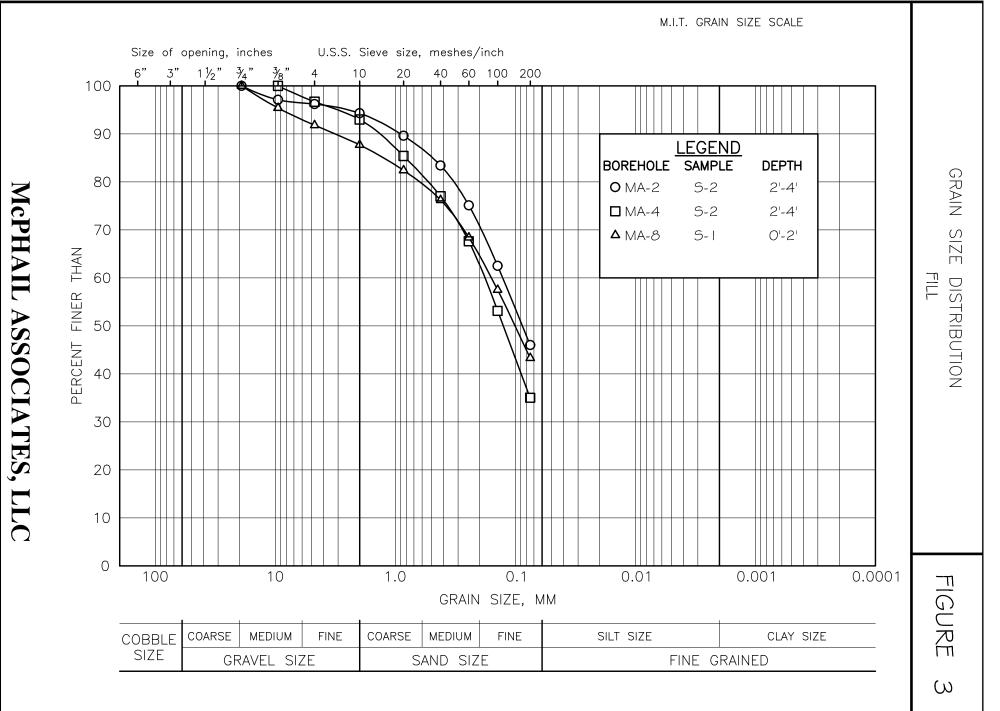
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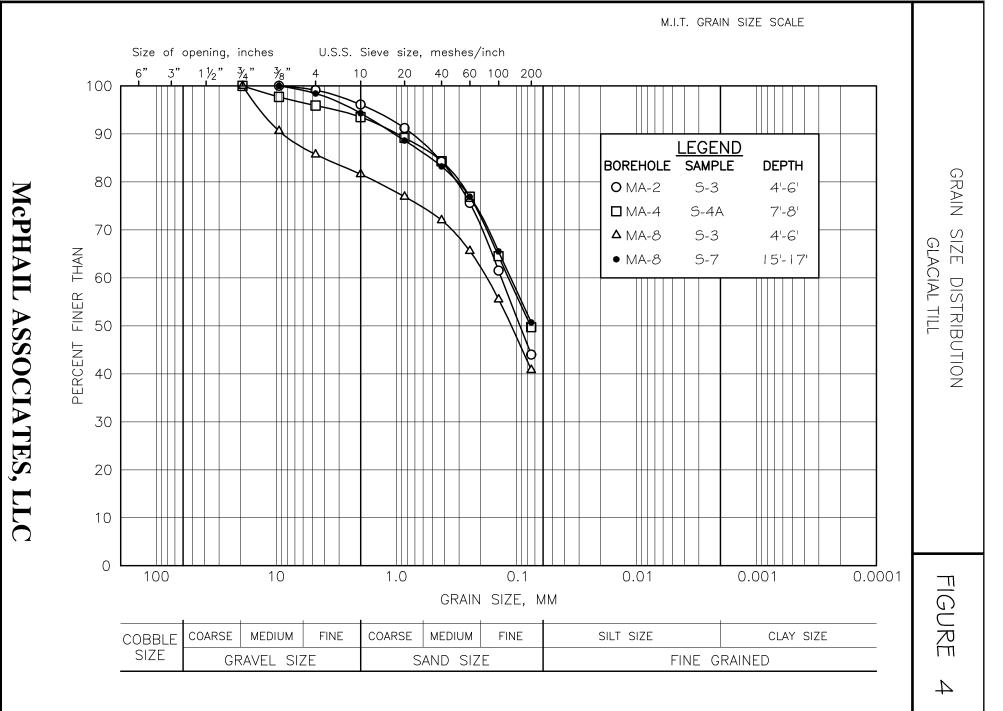






## PROJECT No. 6743





## PROJECT No. 6743



**APPENDIX A:** 

LIMITATIONS



## LIMITATIONS

This preliminary report has been prepared on behalf of and for the exclusive use of Finegold Alexander Architects for specific application to the proposed new Leicester Middle School in Leicester, Massachusetts in accordance with generally accepted soil and geotechnical engineering practices. No other warranty, expressed or implied, is made.

The recommendations contained in this report are for preliminary pricing and design purposes only. Final subsurface exploration program and foundation engineering analyses will be required for the design and construction of the proposed project. In the event that any changes in nature, design, or location of the proposed construction are planned, the conclusions and recommendations contained in this report should not be considered valid unless the changes are reviewed and conclusions of this report modified or verified in writing by McPhail Associates.

The preliminary analyses and recommendations presented in this report are based upon the data obtained from the preliminary subsurface explorations performed at the approximate locations indicated to McPhail. If variations in the nature and extent of subsurface conditions between the widely spaced explorations become evident during the course of construction, it will be necessary for a re-evaluation of the recommendations of this report to be made after performing on-site observations during the construction period and noting the characteristics of any variations.



### **APPENDIX B:**

## BORING LOGS MA-1 THROUGH MA-8 PREPARED BY MCPHAIL ASSOCIATES, LLC

Projec Locat City/S	ion:	70					Date	⊭: Started: Finished	3-6-1		Gr	Boring MA	-1	ions
Driller/ Logged	Helper 1 By/Re	: Bret	t/Donnie d <b>By:</b> K t): 967.5	Ca K. Seaman Sa	mpler S	mmer (l ize/Type	bs)/Drop 9: 24" Sp (Ibs)/Dro	<b>) (in):</b> N/A Nit Spoon N <b>p (in):</b> 140L	.B/30"		Date 3-6-19	Depth 6.0	Elev. 961.5	Notes
Depth (ft)	Elev. (ft)	Symbol	Depth/EL to Strata Change (ft)	Stratum	N-Value RQD	No.	Samp Pen. /Rec. (in)	le Depth (ft)	Blows/6" Min/ft			le Descrip Boring Not		
	967				46	S1	12/11	0.0-1.0	21 25	Dense, light	brown to orange,	SAND, trace s	ilt. (Fill)	
- 1 -	- 966				76	S1A	12/11	1.0-2.0	41	Very dense,	, orange to gray, G	RAVELLY SA	ND, some sil	t. (Fill)
- 2 -	- 965 - 964			FILL	40	S2	24/8	2.0-4.0	35 17 22 18	Dense, brow	wn to gray, SILTY S	SAND and GR	AVEL. (Fill)	
- 4 -	- 963 - 962		4.0 / 963.5	HISTORIC TOPSOIL/SUBSOIL	9	S3	24/10	4.0-6.0	19 3 3 6	Loose, black (Historic To	k-brown, SAND SI psoil/Subsoil)	LT, trace grav	el, with some	root matter.
- 6 - - 7 -	- 961 - 960		6.0 / 961.5	5	33	S4	24/4	6.0-8.0	15 22 18 15 17	Dense, gray	/, SILTY SAND, sc	ome gravel. (G	acial Till)	
- 8 -	- 959 - 958				10	S5	24/10	8.0-10.0	10 5 5 9	Compact, br Till)	rown to gray-browr	n, SILT and SA	ND, some gr	avel. (Glacial
- 10 - - 11 -	- 957 - 956				16	S6	24/12	10.0-12.0	10 10 6 5	Compact, br Till)	rown to gray-browr	n, SILT and SA	ND, some gr	avel. (Glacial
- 12 - - 13 - - 14 -	- 955 - 954			GLACIAL TILL	18	S7	24/12	12.0-14.0	7 7 11 14	Compact, br Till)	rown to gray-browr	n, SILT and SA	ND, some gr	avel. (Glacial
- 15 - - 16 - - 17 - - 18 - - 19 -	- 953 - 952 - 951 - 950 - 949 - 948		20.0 / 947.	5										
- 21 - - 22 -	- 947 - 946			GLACIAL TILL	28	S8	24/6	20.0-22.0	9 12 16 18	Compact, gr	ray, SILT and SAN	D, some grave	el. (Glacial Ti	1)
-	- 945	0:03 0:03												
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8-15 15-30 >30	0	STII V.ST HAF	IFF	Neather: Partly Cloudy								Pag	e 1 of 2	2

Projec Locat City/S	ion:	70		/liddle School w Avenue MA				#: Started: Finished	3-6-1		Boring No. <b>MA-1</b>
Driller/ Loggec	Helper I By/Re	: Bret	t/Donnie <b>d By:</b> K t): 967.5	. Seaman S	Sampler S	mmer (l ize/Type	<b>bs)/Drop</b> 9:24"Sp	<b>(in):</b> N/A	-B/30"		Groundwater Observations Date Depth Elev. Notes 6-19 6.0 961.5
Depth (ft)	Elev. (ft)	Symbol	Depth/EL to Strata Change (ft)	Stratum	N-Value RQD	No.	Samp Pen. /Rec. (in)	Depth (ft)	Blows/6" Min/ft		Sample Description and Boring Notes
24 - 25 - 26 - 27 - 28 - 29 - 30 - 31 -	- 944 - 943 - 942 - 941 - 940 - 939 - 938 - 937		<u>27.0 / 940.</u>	GLACIAL TILL 5 Bottom of boring at 27 feet below ground surface.	18 v	S9	24/2	25.0-27.0	5 9 9 9	Compact, gray, SILT	and SAND, trace gravel. (Glacial Till)
32 - 33 - 34 - 35 - 36 - 37 - 38 -	<ul> <li>936</li> <li>935</li> <li>934</li> <li>933</li> <li>932</li> <li>931</li> <li>930</li> <li>929</li> </ul>										
<ul> <li>39</li> <li>40</li> <li>41</li> <li>42</li> <li>43</li> <li>44</li> <li>45</li> </ul>	- 928 - 927 - 926 - 925 - 924 - 923										
BLOWS 0-4 4-10 10-3( 30-5( >50 CC BLOWS 2-4 4-8	) 0 DHESIVI /FT. C	DENS V.LOC LOOS COMP/ DENS V.DEN SOILS ONSIS V.SC SOIF	ITY ISE ACT SE ISE ISE TENCY FT FT M	SOIL COMPONENT DESCRIPTIVE TERM "TRACE" "SOME" "ADJECTIVE" (eg SANDY, SI "AND"		PORTIOI 0-1( 10-2 20-3 35-5	0% 5%	COM WHIC 25% CLAS	PONENTS CH COMPF OF THE T SSIFIED AS	ING THREE SEACH OF RISE AT LEAST OTAL ARE S"A O MIXTURE OF"	MCPHAIL 2569 MASSACHUSETTS AVENUL CAMBRIDGE, MA 02140 TEL: 617-868-1420 FAX: 617-868-1423
8-15 15-30 >30	0	STII V.ST HAF	IFF	Veather: Partly Cloudy							Page 2 of 2

Projec Locat City/S	ion:	70		<i>l</i> iddle School w Avenue MA				#: Started: Finished	3-6-1		N	ing No. <b>IA-2</b>	
Driller/I Logged	Helper: I By/Re	Bret	t/Donnie d <b>By:</b> K t): 991.8	Seaman Sa	mpler S	mmer (l ize/Type	bs)/Drop 9: 24" Sp (Ibs)/Dro	<b>o (in):</b> N/A blit Spoon b <b>p (in):</b> 140L	.B/30"		Groundw Date De	ater Observ oth Elev.	
Depth (ft)	Elev. (ft)	Symbol	Depth/EL to Strata Change (ft)	Stratum	N-Value RQD	No.	Samp Pen. /Rec. (in)	le Depth (ft)	Blows/6" Min/ft		Sample Dea and Boring		
1 -	- 991 - 990	$\bigotimes$			17	S1	24/20	0.0-2.0	15 11 6 9	Compact, brown, \$	SAND, some silt, tra	ce root matter a	nd gravel. (Fill)
2 -	- 989 - 988		4.0 / 987.8	FILL	28	S2	24/22	2.0-4.0	14 13 15 15	Compact, brown, \$	SILT and SAND, trac	e gravel. (Fill)	
4 - 5 -	- 987 - 986				14	S3	24/18	4.0-6.0	4 6 8 12	Compact, brown to	o tan, SILT and SAN	D, some gravel.	(Glacial Till)
6 - 7 - 8 -	- 985 - 984				20	S4	24/16	6.0-8.0	7 9 11 10	Compact, brown to	o tan, SAND, some s	ilt, trace gravel	(Glacial Till)
8 - 9 - 10 -	- 983 - 982			GLACIAL TILL	20	S5	24/20	8.0-10.0	6 7 13 20	Compact, brown to	o tan, SAND, some s	ilt and gravel. (C	Glacial Till)
11 - 12 -	- 981 - 980												
13 - 14 -	- 979 - 978		14.0 / 977.8		51	S6	24/10	12.0-14.0	16 25 26 32	Very dense, gray,	SILTY SAND, some	gravel. (Glacial	111)
15 - 16 -	- 977 - 976			Bottom fo borehole 14' below ground surface.									
17 - 18 -	- 975 - 974												
19 -	- 973 - 972												
20 - 21 - 22 -	- 971 - 970												
	- 969		_										
BLOWS 0-4		DENS V.LOO	ITY ISE	SOIL COMPONENT	PRO	PORTIO	N OF TOT						
4-10 10-30 30-50 >50		LOOS COMPA DENS V.DEN	ACT SE ISE	"TRACE" "SOME" "ADJECTIVE" (eg SANDY, SIL' "AND"	TY)	0-10 10-2 20-3 35-5	0% 5%	WHIC 25% ( CLAS	CH COMPR OF THE TO SIFIED AS	EACH OF RISE AT LEAST OTAL ARE S "A MIXTURE OF	N	ICPHA ISOCIATES	
CC BLOWS/ <2 2-4 4-8	HESIVE		TENCY N DFT N T	lotes: lo groundwater observed.							2269 MAS CAM TE	IL ASSOCIA SACHUSET BRIDGE, MA L: 617-868 X: 617-868	TS ÁVENUE A 02140 -1420
8-15 15-30 >30	D	STIF V.ST HAF	=F IFF	Veather: Clear								Page 1 o	f 1

Projec Locat City/S	ion:	70		liddle School v Avenue MA				≮: Started: Finished	3-5-1			Boring	-3	
Driller/ Logged	Helper I By/Re	: Bret eviewe	cal Drilling tt/Donnie <b>d By:</b> C <b>t):</b> 987.5	Ca Miller Sa	mpler S	mmer (l ize/Type	bs)/Drop 9: 24" Sp (Ibs)/Dro	o <b>(in):</b> N/A Ilit Spoon Op <b>(in):</b> 140L	_B/30"	-	Gro Date 3-5-19	undwater Depth 6.0	Observat Elev. 981.5	ions Notes
Depth (ft)	Elev. (ft)	Symbol	Depth/EL to Strata Change (ft)	Stratum	N-Value RQD	No.	Samp Pen. /Rec. (in)	le Depth (ft)	Blows/6" Min/ft			e Descrip Boring Not		
- 1 -	- 987 - 986				24	S1	18/16	0.0-1.5	13 13 11		k brown, SILT and			
- 2 -	- 985 - 984		4.0 / 983.5	FILL	20 19	S1A S2	6/6 24/23	1.5-2.0 2.0-4.0	10 5 9 10 14		t brown, SAND ar t brown/orange-bi			,
- 4 - - 5 -	- 983 - 982		4.07 303.3		18	S3	24/24	4.0-6.0	6 8 10 12	Compact, light	t brown, SILTY S/	AND, some gr	avel. (Glacia	I Ti∥)
- 6 - - 7 -	- 981 - 980				28	S4	24/16	6.0-8.0	10 13 15 17	Compact, light (Glacial Till)	t brown/orange-bi	rown, SILTY S	AND, some	gravel.
· 8 - · 9 - · 10 -	- 979 - 978			GLACIAL TILL	30	S5	24/20	8.0-10.0	5 12 18 22	Dense, orang (Glacial Till)	e-brown, SILTY S	GAND, some g	ravel, occas	ional cobbles
· 11 -	- 977 - 976 - 975													
13 - 14 -	- 974 - 973		14.7 / 972.8		24	S6	20/11	13.0-14.7	8 16 8	(Glacial Till)	e-brown, SILTY S fusal at 14.7' belo	-		ional cobbles
15 - 16 - 17 -	- 972 - 971 - 970			Bottom of borehole 14.7' below ground surface.	<u> </u>				100/2"					
18 - 19 -	- 969 - 968													
20 - 21 - 22 -	- 967 - 966													
	- 965 RANULA			SOIL COMPONENT								_		_
BLOWS 0-4 4-10 10-30 30-50 >50 CC BLOWS <2 2-4 4-8	/FT. D D DHESIV /FT. C	DENS V.LOC LOOS COMP/ DENS V.DEN E SOILS SONSIS V.SC SOI FIR	ITY DSE GE ACT GE ISE S TENCY N DFT F1 M	DESCRIPTIVE TERM "TRACE" "ADJECTIVE" (eg SANDY, SIL" "AND" otes: rost from 0'-1.25' below ground :	ΓΥ)	0-10 0-10 10-2 20-3 35-5	0% 5%	COM WHIC 25% CLAS	PONENTS CH COMPF OF THE T SSIFIED AS	ING THREE EACH OF RISE AT LEA OTAL ARE 3 "A D MIXTURE (	AST OF" M		HUSETTS	ÁVENUE 2140 20
8-15 15-30 >30	5	STII V.ST HAF	IFF	/eather: Clear								Pag	e 1 of 1	1

Projec Locat City/S	ion:	70		Middle School w Avenue MA				♯: Started: Finished	3-5-1			Boring	-4	
Driller/ Loggec	Helper 1 By/Re	: Bret eviewe	tt/Donnie d <b>By:</b> C t): 991.6	Ca C. Miller Sa	mpler S	mmer (l ize/Type	<b>bs)/Drop</b> 9: 24" Sp	9 (in): N/A Nit Spoon Op (in): 140L	.B/30"		Gro Date 3-5-19	Depth 8.0	Observat Elev. 983.6	ions Notes
Depth (ft)	Elev. (ft)	Symbol	Depth/EL to Strata Change (ft)	Stratum	N-Value RQD	No.	Samp Pen. /Rec. (in)	le Depth (ft)	Blows/6" Min/ft			ble Descrip Boring Not		
- 1 -	- 991 - 990		0,		25	S1	24/22	0.0-2.0	13 13 12 12	Compact, gr	ay brown/brown S	GILTY SAND, S	ome gravel. (	Fill)
- 2 -	- 989 - 988			FILL	16	S2	24/17	2.0-4.0	7 9 7 5	Compact, lig	ht brown, SILTY S	SAND, trace gr	avel. (Fill)	
4 - 5 -	- 987 - 986				5	S3	24/16	4.0-6.0	4 3 2 2	Loose, brow	m, SILTY SAND, t	race gravel. (F	ill)	
- 6 - - 7 -	- 985		7.0 / 984.6	; ;	7	S4	12/8	6.0-7.0	2 5		m, SILTY SAND,			(Ola-i-1 T**
- 8 -	- 984 - 983				53 70	S4A S5	12/11 24/18	7.0-8.0	12 41 11 35 35	Mottling at a	orange brown, Sl pproximately 7' be orange brown, Sl	elow ground sur	face.	
- 10 - - 11 - - 12 -	- 982 - 981 - 980			GLACIAL TILL	122	S6	19/12	11.0-12.6	59 90 71 51	cobbles. (Gla			-	
· 13 - · 14 - · 15 - · 16 - · 17 - · 18 - · 19 -	- 979 - 978 - 977 - 976 - 975 - 974 - 973	<u>&gt;`o.f.X</u> a	<u>12.6 / 979.</u>	Bottom of borehole 12.6' below ground surface.	<u>\</u>				100/1"	ground surfa	al 11' below groun	u surrace. spir	sponreus	
· 20 - · 21 - · 22 -	- 972 - 971 - 970 - 969													
BLOWS 0-4 4-10 10-30 30-50 >50 CC BLOWS <2	RANULA /FT. 0 0 0 DHESIV /FT. C	DENS V.LOC LOOS COMP/ DENS V.DEN E SOILS CONSIS	ITY DSE GE ACT GE ISE S TENCY DFT	SOIL COMPONENT DESCRIPTIVE TERM "TRACE" "SOME" "ADJECTIVE" (eg SANDY, SIL" "AND"		PORTION 0-10 10-2 20-3 35-5	0% 5%	COM WHIC 25% ( CLAS	PONENTS CH COMPF OF THE T SSIFIED AS	ING THREI SEACH OF RISE AT LE OTAL ARE S "A O MIXTURE	AST E OF"	McClasso McPhall AS 9 MASSAC CAMBRID TEL: 6	HUSETTS	ÁVENUE 2140
2-4 4-8 8-15 15-3( >30	5	SOI FIR STII V.ST HAF	M FF IFF	Veather: Clear								FAX: 6	e 1 of 1	23

Projec Locat City/S	ion:	70		<i>l</i> iddle School v Avenue MA				♯: Started: Finished	3-5-1			Boring MA	-5	
Driller/ Logged	Helper: I By/Re	: Bret viewe	tt/Donnie d By: C t): 996.3	. Miller Sa	ampler Si	mmer (l ize/Type	<b>bs)/Drop</b> ə: 24" Sp ( <b>Ibs)/Dro</b>	<b>op (in):</b> 140L	.B/30"	-	Gro Date 3-5-19	Undwater Depth 8.0	Observa Elev. 988.3	tions Notes
Depth (ft)	Elev. (ft)	Symbol	Depth/EL to Strata Change (ft)	Stratum	N-Value RQD	No.	Samp Pen. /Rec. (in)	le Depth (ft)	Blows/6" Min/ft			e Descrip Boring Not		
1 -	- 996 - 995				19	S1	24/24	0.0-2.0	13 12 7 8	Compact, brow	wn, SILTY SAND	, some gravel	, trace root r	natter. (Fill)
2 -	- 994 - 993			FILL	18	S2	24/22	2.0-4.0	7 7 11 9	Compact, gray	y brown/orange b	rown, SILTY S	SAND, trace	gravel. (Fill)
4 -	- 992 - 991		5.0 / 991.3		20	S3	24/24	4.0-6.0	8 9 11 14	Compact, gra	y brown, SILTY S	AND, some g	ravel. (Fill)	
6 -	- 990 - 989				39	S4	24/24	6.0-8.0	19 19 20 22	Dense, gray b	Drown, SILTY SAN	ND, some grav	vel. (Glacial	Till)
8 - 9 -	- 988 - 987			GLACIAL TILL	30	S5	24/24	8.0-10.0	11 12 18 26	Dense, gray b	prown, SILTY SAN	ND, some grav	/el. (Glacial	Till)
10 -	- 986 - 985		11.5 / 984.8	Bottom of borehole 11.5' below	114/11"	S6	17/12	10.0-11.4	11 14 100/5"	occasional co	ray/orange browr bbles. (Glacial Ti 11.5' below grour	II)	D, some gra	vel, with
12 - 13 -	- 984 - 983			ground surface.										
14 - 15 -	- 982 - 981													
16 - 17 -	- 980 - 979													
18 - 19 -	- 978 - 977													
20 - 21 -	- 976 - 975													
22 - GB	- 974 RANULA		s											
BLOWS 0-4 4-10 10-30 30-50 >50	/FT.   	DENS V.LOC LOOS COMP/ DENS V.DEN	ITY DSE SE ACT SE ISE	SOIL COMPONENT DESCRIPTIVE TERM "TRACE" "SOME" "ADJECTIVE" (eg SANDY, SIL "AND"		PORTIOI 0-1( 10-2 20-3 35-5	0% 5%	COMI WHIC 25% ( CLAS	PONENTS CH COMPF OF THE T SSIFIED AS	ING THREE EACH OF RISE AT LEA OTAL ARE S "A O MIXTURE (	AST		PHA CIATES, I	
CC BLOWS <2 2-4 4-8	HESIVE		TENCY N DFT FT	lotes:							м		HUSETTS	S ÁVENUE 2140 420
8-15 15-30 >30	D I	STII V.ST HAF	FF	Veather: Clear								Pag	e 1 of	1

Projec Locat City/S	ion:	70		∕liddle School w Avenue MA				#: Started: Finished	3-6-1			Boring MA		
Driller/ Logged	Helper I By/Re	: Bret	t/Donnie <b>d By:</b> K t): 993.2	. Seaman Sa	ampler S	mmer (l ize/Type	bs)/Drop 9: 24" Sp (Ibs)/Dro	<b>op (in):</b> 140L	B/30"		Gro Date 3-6-19	undwater Depth 6.0	Observa Elev. 987.2	tions Notes
Depth (ft)	Elev. (ft)	Symbol	Depth/EL to Strata Change (ft)	Stratum	N-Value RQD	No.	Samp Pen. /Rec. (in)	le Depth (ft)	Blows/6" Min/ft			e Descrip Boring Not		
- 1 -	- 993 - 992	$\bigotimes$			14	S1	24/14	0.0-2.0	12 7 7 6	Compact, brown	n, SAND, some	silt, trace gra	vel, brick an	d ash. (Fill)
- 2 -	- 991 - 990			FILL	17	S2	24/12	2.0-4.0	5 8 9 21	Compact, gray,	SAND, some cl	lay, silt, and g	ravel. (Fill)	
- 4 -	- 989 - 988		5.0 / 988.2		- 34	S3	24/14	4.0-6.0	10 10 24 23	Dense, gray, SII	LTY SAND, sor	ne clay and g	ravel. (Fill)	
- 6 -	- 987 - 986				41	S4	24/12	6.0-8.0	18 18 23 23	Dense, gray, SII	LTY SAND, sor	ne clay and g	avel. (Glaci	al Till)
- 8 -	- 985 - 984			GLACIAL TILL										
- 10 - - 11 -	- 983 - 982		12.0 / 981.2	2	55	S5	24/6	10.0-12.0	29 30 25 29	Very dense, gra	y, SILTY SAND	), some clay a	nd gravel. (0	Glacial Till)
- 12 - - 13 -	- 981 - 980	- 41 -		Bottom of borehole 12' below ground surface.										
· 14 -	- 979 - 978													
· 16 -	- 977 - 976													
· 18 -	- 975 - 974													
- 20 - - 21 -	- 973 - 972													
- 22 - GR	- 971	R SOIL	s											
BLOWS 0-4 4-10 10-30 30-50 >50	/FT.	DENSI V.LOO LOOS COMPA DENS V.DEN	TY SE SE ACT SE SE	SOIL COMPONENT DESCRIPTIVE TERM "TRACE" "SOME" "ADJECTIVE" (eg SANDY, SIL "AND"		0-10 0-10 10-2 20-3 35-5	0% 5%	COMF WHIC 25% ( CLAS	PONENTS CH COMPF OF THE T SIFIED AS	ING THREE EACH OF RISE AT LEAS OTAL ARE S "A D MIXTURE O	F" M	ASSOC	SOCIATI	ES, LLC
<2 2-4 4-8		V.SO SOF FIR	PFT FT M	IUI03.							2209	CAMBRID TEL: 6		)2140 420
8-15 15-30 >30		STIF V.ST HAF	IFF	Veather: Clear								Pag	e 1 of	1

Projec Locat City/S	ion:	70		<i>l</i> iddle School v Avenue MA				♯: Started: Finished	3-6-1			Boring MA		
Contra Driller/ Loggec	ctor: Helpei d By/Re	Technie :: Bre eviewe	cal Drilling tt/Donnie <b>d By:</b> K i <b>t):</b> 1000.0	y Services Ca Ca . Seaman Sa	mpler S	mmer (l ize/Type	<b>bs)/Drop</b> <b>ə:</b> 24" Sp	<b>) (in):</b> N/A blit Spoon <b>op (in):</b> 140L	.B/30"		Grou Date 3-6-19	Depth 8.0	Observat Elev. 992.6	ions Notes
Depth (ft)	Elev. (ft)	Symbol	Depth/EL to Strata Change (ft)	Stratum	N-Value RQD	No.	Samp Pen. /Rec. (in)	le Depth (ft)	Blows/6" Min/ft			e Descript oring Not		
1 -	- 1000 - 999		- <del>0</del> 2.0 / 998.6	FILL	33	S1	24/20	0.0-2.0	15 16 17 12	Dense, brown, Sl	ILT and SAND,	, some gravel	, trace root n	natter. (Fill)
2 - 3 -	- 998 - 997		2.07 998.0		20	S2	24/20	2.0-4.0	7 9 11 11	Compact, brown	to gray, SILTY	SAND, some	gravel. (Gla	cial Till)
4 - 5 - 6 - 7 - 8 -	- 996 - 995 - 994 - 993			GLACIAL TILL										
9 - 10 - 11 -	- 992 - 991 - 990		11.0 / 989.6	6	51	S3	24/4	8.0-10.0	19 24 27 30	Very dense, gray	/, SILTY SAND	, some grave	l, trace clay.	(Glacial Till
12 - 13 - 14 -	- 989 - 988 - 987 - 986				35	S4	24/14	13.0-15.0	10 16 19	Dense, gray, SIL	T and SAND, t	race clay and	gravel. (Gla	cial Till)
15 - 16 - 17 -	- 985 - 984			GLACIAL TILL	22	S5	24/8	15.0-17.0	20 8 10 12 16	Compact, gray, S	SILT and SAND	), trace clay a	nd gravel. (G	Glacial Till)
18 - 19 -	- 983 - 982		19.0 / 981.6	8	41	S6	24/20	17.0-19.0	16 19 22 23	Dense, gray, SIL	T and SAND, t	race clay and	gravel. (Gla	cial Till)
20 - 21 - 22 -	- 981 - 980 - 979 - 978			Bottom of borehole 19' below ground surface.										
BLOWS 0-4 4-10 10-30 30-50 >50	RANULA //FT. 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	AR SOIL DENS V.LOC LOO COMP DENS V.DEN E SOIL CONSIS V.SC SO	ITY DSE GE ACT GE ISE S TENCY DFT	SOIL COMPONENT DESCRIPTIVE TERM "TRACE" "SOME" "ADJECTIVE" (eg SANDY, SIL" "AND"		PORTIO 0-1( 10-2 20-3 35-5	0% 5%	COM WHIC 25% CLAS	PONENTS CH COMPF OF THE T SSIFIED AS	ING THREE 5 EACH OF RISE AT LEAS OTAL ARE 5 "A 0 MIXTURE OF	=" Mo 2269	CPHAIL AS MASSAC CAMBRID TEL: 6	HUSETTS GE, MA 0 17-868-14	ÁVENUE 2140 20
4-8 8-15 15-30 >30	5	FIR STI V.ST HAF	M FF IFF	Veather: Clear									17-868-14 e 1 of 1	

Projec Locat City/S	ion:	70		Middle School w Avenue MA				#: Started: Finished	3-6-1	-		Boring	-8	<b>1</b>
Driller/ Logged	Helper I By/Re	: Bret	tt/Donnie <b>d By:</b> K t <b>t):</b> 1001.	C Seaman S	ampler S	mmer (l ize/Type	lbs)/Drop e: 24" Sp (lbs)/Dro	op (in): 140L	.B/30"		Date	oundwater Depth	Observa Elev.	Notes
Depth (ft)	Elev. (ft)	Symbol	Depth/EL to Strata Change (ft)	Stratum	N-Value RQD	No.	Samp Pen. /Rec. (in)	le Depth (ft)	Blows/6" Min/ft			ble Descrip Boring Not		
- 1 -	- 1000	$\bigotimes$			25	S1	24/20	0.0-2.0	13 13 12 10	Compact, bro	own, SILT and SA	AND, some gra	vel. (Fill)	
- 2 -	- 999 - 998		4.0 / 997.0	FILL	31	S2	24/14	2.0-4.0	13 14 17 37		rn, SAND, some s al at 4.3' below gro		[Fill)	
- 4 -	- 997 - 996				26	S3	24/10	4.0-6.0	10 12 14 12	Compact, bro (Glacial Till)	own to gray, SILT	and SAND, so	ome gravel, t	race clay.
- 6 - - 7 - - 8 -	- 995 - 994 - 993				30	S4	24/20	6.0-8.0	25 14 16 17	Compact to o (Glacial Till)	dense, gray, SILT	and SAND, so	ome gravel, t	trace clay.
- 9 - - 10 -	- 993 - 992 - 991			GLACIAL TILL										
· 11 - · 12 -	- 990 - 989				28	S5	24/14	11.0-13.0	7 11 17	Compact, gra Till)	ay-brown, SILT a	nd SAND, trac	e clay and g	avel. (Glacial
13 - 14 -	- 988 - 987		13.0 / 988.		43	S6	24/12	13.0-15.0	17 15 16 27 12	Dense, gray,	, SILT and SAND	, trace clay and	d gravel. (Gla	acial Till)
	- 986 - 985		17.0 / 984.	GLACIAL TILL	34	S7	24/20	15.0-17.0	13 15 19 21	Dense, gray,	, SILT and SAND	, trace clay and	d gravel. (Gla	acial Till)
	- 984 - 983			Bottom of borehole 17' below ground surface.										
20 -	- 982 - 981													
	- 980 - 979													
BLOWS 0-4 4-10 10-30 30-50 >50	) )) )) HESIV /FT. C	DENS V.LOC LOOS COMP DENS V.DEN E SOILS	ITY DSE GE ACT GE ISE S TENCY FT	SOIL COMPONENT DESCRIPTIVE TERM "TRACE" "SOME" "ADJECTIVE" (eg SANDY, SII "AND" Jotes: Io groundwater observed.		0-11 0-11 10-2 20-3 35-5	20% 35%	COM WHIC 25% CLAS	PONENTS CH COMPF OF THE T SIFIED AS	ING THREE EACH OF RISE AT LE OTAL ARE S "A O MIXTURE	AST	MCPHAIL AS CAMBRID TEL: 0	HUSETT	S ÁVENUE )2140
2-4 4-8 8-15 15-30 >30		FIR STII V.ST HAF	M FF IFF	Veather: Clear								FAX: 6	e 1 of	423